

March 7, 2024

Ricky Vehar
Environmental Specialist 2
Engineering and Infrastructure Section
Division of Drinking and Ground Waters

Subject: Ashville Village PWS
Plan Correspondence – General Plan
Package No. DDAWG - 662

Dear Ms. Vehar:

The following is our response to the above reference project and your letter dated December 14, 2023. Our responses follow the letter format.

Future water demand projects have been altered from the initial General Plan submission. Development opportunities are expected to slow causing future demand to decrease. The new 20-year maximum day demand has been adjusted to 1.25 MGD. The plant would be designed with an initial rated capacity of 1.29 MGD. There is another 0.16 MGD capacity available from the Earnhart Hills connection. An emergency connection is available to South Bloomfield too.

Required:

| Comment ID | Page Number | Response |
|------------|-------------|---|
| DDAGW-1 | 31 | A present worth analysis is included in Section 8 for the two feasible alternatives – Option 1, New WTP; and Option 2, New WTP with Aeralator. Regionalization and rehabbing the existing facility are not feasible as explained in Section 5. Monetary and Non-monetary considerations are described in Section 5. |
| DDAGW-2 | 24 | Ashville, Circleville and Earnhart Hill Water and Sewer District met on January 29, 2024 to discuss a regionalization approach. After considering the possibility, this was deemed to not be a viable alternative. This is discussed in more detailed in Section 5.1. Because the alternative is not considered viable a full present worth estimate was not completed. |
| DDAGW-3 | 22 | Future water demands have been amended. See new projections in General Plan. |
| DDAGW-4 | 36 | The Village intends to pursue a 20-year loan for this project. |

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| DDAGW-5 | 32 | Tables have been updated with the revised demand capacities. |
| DDAGW-6 | 9 | Historical water demands have been updated. |
| DDAGW-7 | 20 | Ashville's growth estimates have been revised. |
| DDAGW-8 | 32 | With the revised future water demands the capacity of the wells with one out of service is 1200 gpm and meets the demand requirements. |
| DDAGW-9 | 20 | Raw water fluoride levels range between 1.04 and 1.47 mg/L. Fluoride removal is not included in the treatment process so the levels are expected to be in this range in the finished water, forgoing the need to add fluoride. Raw water inorganics have been added to the appendix. |
| DDAGW-10 | 32 | Table has been updated to reflect Equivalent Max-Day Demand. |
| DDAGW-11 | 33 | The projected max day demand has been revised and there is sufficient filtration capacity with one filter cell out of service. |
| DDAGW-12 | 29 | A desktop corrosion control evaluation will be completed during the design phase. Corrosion Control Treatment Recommendation Forms (CCTR Forms) will be completed during design and submitted for approval. After the new plant goes online, the Village will complete the required water quality parameter sampling and additional lead and copper sampling. |
| DDAGW-13 | 6 | Values have been updated and shown in ug/L. |
| DDAGW-14 | 6 | In the original General Plan is was listed as Well 5, but this has been corrected to Well 6. Well 5 was never put into service. Well 6 will be used in place of 2 and 3. Wells 2 and 3 will be abandoned. Their capacities are 200 gpm and 240 gpm respectively. |
| DDAGW-16 | 9 | A section on the distribution system has been added, Section 3.6.12. |
| DDAGW-17 | 34 | The Village is in the process of expanding the WWTP to coincide with the expansion of the WTP. The WWTP is planning to double capacity from the current 0.8 mgd capacity to 2.0 mgd capacity. This is included in section 4.1 of the General Plan. |

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Plan Sheet Comments:

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| DDAGW-15 | 11 | An existing process flow diagram has been added to Section 3.6.2. The new process flow diagram has been updated in the Appendix. |
| DDAGW-18 | 5 | Appendix B has been updated to only include the recommended building layout. |
| DDAGW-19 | 11 | Backflow prevention from filter and softener waste will be provided via an air gap. The process flow diagram in Appendix C has been updated to show the air gaps. |

The Report has been revised to reflect these comments and is included for your review. A project schedule has been added to Section 6.

Sincerely,

JONES & HENRY ENGINEERS, LTD.



Jake Meinerding, PE
Principal, Director of Water Treatment

C: Franklin Christman, Village of Ashville
Maria Lucente, P.E. Environmental Manager, DDAGW-CO

Enc: Revised General Plan